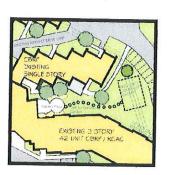
PLANNED DEVELOPMENT DISTRICT AVALON SENIOR CAMPUS

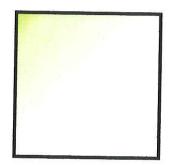
GENERAL DEVELOPMENT PLAN SPECIFIC IMPLEMENTATION PLAN

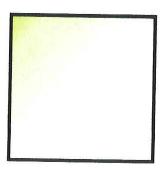
FEBRUARY 19th, 2007 REVISED, MARCH 6th, 2007













2875 FISH HATCHERY ROAD FITCHBURG, WISCONSIN

PLANNED DEVELOPMENT DISTRICT

W/ General Implementation Plan and Specific Implementation Plan (PDD - GIP- SIP)

Table of Contents

		Page
	ANNED DEVELOPMENT DISTRICT GENERAL PLEMENTATION PLAN (PDD - GIP)	
1.	INTRODUCTION	1
2.	PROJECT DESCRIPTION	. 1
	Existing Conditions	1
	Proposed Development	2
3.	SITE PLANS AND LOCATION MAP (REFERS TO APPENDIX B)	2
4.	RATIONALE FOR PLANNED DEVELOPMENT DISTRICT ZONING	2
5.	SOCIAL, ENVIRONMENTAL AND ECONOMIC IMPACTS	3
6.	GENERAL DEVELOPMENT PLAN	4
	(a) Public and private roads, driveways and parking facilities	4
	(b) Land uses and size, arrangement and location of lots and proposed buildings or groups of buildings.	5
	(c) The types, size and location of structures.	5
	(d) General Utility Plan	5
	(e) Recreational and Open Space Areas	5
	(f) General landscape treatment plan.	6
	(g) Statistical data	6
	Land Use Chart	7
	Organizational Structure	8

7. PLANNED DEVELOPMENT DISTRICT SPECIFIC IMPLEMENTATION PLAN (PDD - SIP)

9

8. APPENDICES

Appendix A: Contacts

Appendix B: Maps

2.1 General Development Plan

2.2 Conceptual Landscape and Open Space Plan

2.3 Current Conditions Plan

2.4 Parcel Map

2.5a Certified Survey Map (overall)

2.5b Certified Survey Map Detail

2.5c Certified Survey Map Detail

2.6 Condominium Map

2.7 Utility Map

2.8 Topography Map

2.9 Tree Location Map

2.10 Existing Drainage Plan

2.11 Proposed Drainage Plan

2.12 Preliminary Stormwater Management Plan

Appendix C: Preliminary Storm Water Management Calculations

PLANNED DEVELOPMENT DISTRICT GENERAL IMPLEMENTATION PLAN (PDD - GIP)

1. INTRODUCTION

Avalon Senior Care is submitting an application for rezoning from the Residential – Low Density District (R-L) with a C.U.P for a 106-bed senior care facility to Planned Development District – General Implementation Plan (PDD-GIP). A CSM for a new land division is being submitted concurrent with the PUD-GIP.

In addition, Specific Implementation Plans (SIP's) are also being concurrently submitted for the existing structures, this will enable sale of the properties. No exterior changes are proposed for any of the existing structures.

The Planned Development District provides a regulatory framework to encourage improved environmental design by allowing flexibility in the development of land while ensuring compliance with the basic intent of the Zoning Ordinance and with the City land use plan. The Planned Development District has limited standards and specifications. Developers can propose uses or combination of uses and configurations of intensity and density of development. Through a process of Plan Commission review, public hearing and Common Council review and approval, accompanied by discussions with developers and, as appropriate, with other interested parties, an agreement is reached between the property owner and the City of Fitchburg. The details of this agreement constitute the force and effect as do standard zoning requirements.

2. PROJECT DESCRIPTION

Existing Conditions

The project is designed to create an integrated senior care campus on 10.5 acre site located at 2875 Fish Hatchery Road.

The property consists of five tax parcels, all of which are currently owned by Homeville Fitchburg LLC.

Development on the site currently consists of a Community Based Residential Facility (CBRF) and Residential Care Apartment Complex (RCAC) licensed for 106 beds. The beds are located in a 3-story 42-unit CBRF / RCAC structure and in three separate one-story CBRF structures, one of which is connected by an indoor passageway to the main 42-unit facility. In addition to the senior care units, there are approximately 9,000 square feet of office space in an office wing connected to the 42-unit building. The current office tenants are senior care related agencies.

A CBRF is defined by Wisconsin State Statutes, Section 50.01(1g): "Community Based Residential Facility means a place where 5 or more unrelated adults reside in care which care, treatment or services above the level of room and board but not including nursing care are provided to persons residing in the facility as a primary function of the facility".

A Residential Care Apartment Complex (RCAC) means "a place where five or more adults reside that consists of independent apartments, each of which has an individual lockable entrance and exit, a kitchen, including a stove, and individual bathroom, sleeping and living areas, and that

provides, to a person who resides in the place, not more than 28 hours per week of services that are supportive, personal and nursing services."

Independent Senior Housing is defined in the industry as housing for individuals 55 years or better.

Avalon Senior Care currently has 34 full-time and 12 part-time employees. The maximum number of employees on shift at any given time is 13.

The western frontage (Lot 1) along Fish Hatchery Road and approximately 4 acres on the eastern end of the property (Lot 3) are currently undeveloped. The eastern portion of the property has access from Index Road.

Proposed Development

Avalon Senior Care proposes to develop the site as an integrated senior campus.

The existing 42-unit building, connected office wing, and two of the CBRF buildings are proposed for sale to Ridgeline Management Company, a national organization based in Eugene, Oregon that specializes in the management and operation of assisted living, memory care (Alzheimer's care) and independent retirement communities.

The third existing CBRF building will be converted from a CBRF facility licensed for 32 beds to 16 independent senior housing units.

The frontage parcel along Fish Hatchery Road (Lot 1) is proposed for development as a commercial services / office building focusing on tenants that provide services to seniors or related health services. Examples could include professional offices (Podiatrist; Chiropractic, Physical Therapy).

The eastern parcel (Lot 3), which consists of roughly 4 acres is proposed for development for up to 120 senior housing and/or care units. It is unknown at this point what percentage of the senior units will be independent or care units. No construction on the east lot, proposed lot 3, can occur until Index Road is built and constructed to Post Road and Post Road is connected from Fish Hatchery east to the dead end Post Road in the city of Madison.

3. SITE PLANS AND LOCATION MAP

Site plans and location maps are shown on the attached plan sheets.

4. RATIONALE FOR PLANNED DEVELOPMENT DISTRICT ZONING

The applicant wishes to rezone the property from the R-L District to PDD. The rezoning and new CSM will offer a number of advantages to both the City and the applicant. These include:

- The rezoning and subsequent completion of the campus construction will remedy a
 condition of deterioration at the site. Under prior ownership, the property was going
 through bankruptcy. Little investment had been made in physical improvements to the
 site. The rezoning will enable sale of the existing property to a nationally-known senior
 care organization, which will bring new investment and operational expertise to the
 property.
- 2. The current parcelization in very antiquated and has resulted in convoluted and irrational parcel boundaries that don't conform to the improvements on the site. The current

- parcels do not conform to existing land division and zoning regulations. The PDD-GIP and concurrent CSM will rationalize the parcel boundaries and easements.
- City staff has indicated that the PPD zoning is necessary because the existing buildings
 on the site would not have direct public street access with the proposed CSM. Full
 utilization and development of the frontage lot on Fish Hatchery Road and the eastern lot
 fronting on Index Road (extended) would not be feasible without PDD zoning.
- The setbacks between existing buildings would not conform to the R-L District requirements.
- 5. The proposed new development on the site will be designed in conformance with the design guidelines contained in the Chapter 5 of the North Fish Hatchery Road Plan, which recommends setbacks and design features that typically require PDD or PUD-style zoning, rather than standard Euclidean zoning.

5. SOCIAL, ENVIRONMENTAL AND ECONOMIC IMPACTS

The proposed rezoning and redevelopment of the Avalon Senior Care property will have significant social, environmental and economic benefits to the community which include:

- Over the past 10 years the condition of the property has declined. The prior
 management had allowed the property to enter into bankruptcy proceedings. Aside from
 the construction of the 42-unit building in 1998, there has been very little investment in
 physical improvements to the site. The interior units are in need of renovation.
- The sale of the CBRF and RCAC care units to a nationally-recognized organization will stabilize the properties and result in enhance maintenance and reinvestment.
- The present owner intends to rehabilitate and retrofit the north 32-bed CBRF facility into 16 unit independent living units, which will meet a market demand for senior housing units in Fitchburg.
- The new development within the campus will consist of additional senior housing and care facilities. The development of senior and health care related commercial services in the portion of the property will help form a senior care cluster which is recommended in the North Fish Hatchery Road Plan.
- The addition of up to 120 new senior housing or care units will have very little impact on community facilities or services. The units or beds will not add students to the school district or put pressure on the community's park and recreation system. All of the recreational needs of the residents and patients in the facility will be met by on-site recreational facilities.
- The development is expected to have minimal environmental impacts. The soil
 conditions and topography of the site are suitable for development and the site has been
 identified as a future Retail / Commercial and Residential development site in the North
 Fish Hatchery Road Plan.
- Mature trees on portions of the Fish Hatchery Road frontage site will be removed, but the
 applicant intends to install new replacement canopy trees. Existing vegetation along the
 south and north property lines will be preserved. The portion of the site occupied by the
 existing apple orchard will be protected through a deed restriction on the eastern
 development parcel.

- The development will meet all of the stormwater management and erosion control requirements of the City.
- Over 35 percent of the overall site will be maintained in an open and pervious condition.

6. GENERAL DEVELOPMENT PLAN

The attached plan sheets include the following:

- Sheet Number:
 - 2.1 General Development Plan
 - 2.2 Conceptual Landscape and Open Space Plan
 - 2.3 Current Conditions Plan
 - 2.4 Parcel Map
 - 2.5a Certified Survey Map (overall)
 - 2.5b Certified Survey Map Detail
 - 2.5c Certified Survey Map Detail
 - 2.6 Condominium Map
 - 2.7 Utility Map
 - 2.8 Topography Map
 - 2.9 Tree Location Map
 - 2.10 Existing Drainage Plan
 - 2.11 Proposed Drainage Plan
 - 2.12 Preliminary Stormwater Management Plan

(a) Public and private roads, driveways and parking facilities

The property has approximately 280 feet of frontage on Fish Hatchery Road and 440 feet of frontage on Index Road (extended).

The eastern parcel (Lot 3) will not be developed until Index Road has been extended and utilities are installed within the street right-of-way. (No construction on the east lot, proposed lot 3, can occur until Index Road is built and constructed to Post Road and Post Road is connected from Fish Hatchery east to the dead end Post Road in the city of Madison).

The interior lots occupied by the existing senior care facilities (Lot 2, Units A & B) will be served by a private road extending in a generally east-west direction through the site. The access to the road will be provided by easements. The pavement width of the private road will be 24 feet.

The applicant has provided a parking capacity study indicating that up to 124 parking stalls could be provided for the existing facilities. Because very few of the CBRF and RCAC residents drive, the demand for parking stalls is primarily to serve employees and residents. The maximum employment on site at the peak shift is 13 employees. Accepted parking standards for CBRF/RCAC occupancy is 1 stall per 6 beds, plus 1 stall for each employee, plus 1 stall for each visiting physician. This totals 30 stalls for the remaining 74 beds in the CBRF/RCAC (minus the proposed 16 unit independent senior units on Lot 2A).

The 9,000 square foot of existing office space would potentially require 30 stalls, based on 1 parking stall per 300 SF of building area.

The 16 units of proposed senior independent living units concerted from the 32-bed CBRF facility, (Lot 2, Unit A) are projected to require up 24 parking stalls, based on 1.5 parking stalls per senior unit.

Parking in the proposed senior housing and care unit on the eastern parcel (Lot 3) will be a combination of surface and underground parking. The parking for this facility will be reviewed at the time of a Specific Implementation Plan is submitted for this parcel; although it is acknowledged that a min of 1 stall per unit will be provided underground.

It is unknown at this point if the parking on the commercial / office parcel (Lot 1) on Fish Hatchery Road will be surface, underground, or a combination of the two. The parking for this facility will be reviewed at the time of a Specific Implementation Plan is submitted for this parcel. The site plans show a parking layout based on a full 2 storey building with surface parking only.

Refer to the Land Use Table for Parking information.

(b) Land uses and size, arrangement and location of lots and proposed buildings or groups of buildings.

The land uses will include a combination of senior care and senior independent living and senior and health care related commercial services.

The existing group of senior care facilities will remain unchanged, except for interior improvements. The CBRF and RCAC units (Parcel B) will have new management provided by Ridgelines Management Company.

The existing north CBRF building (Lot 2, Unit A) will be converted to 16 units of senior independent housing.

The new commercial development on Lot 1 is expected to be in one building with minimum of 2 full stories or 3 stories with a footprint of roughly 7,000 square feet (the third story, if used will be stepped back from the front exposure to Fish Hatchery road).

The new senior housing and/or senior care facility with up to 120 units on Lot 3 will be a 2 to 3 story building with underground parking, and it is acknowledged that a min of 1 stall per unit will be provided underground. It is recognized that 120 units, maximum represents a high density, however, due to the relative low impact of this unique segment of the population, we feel the property can support the anticipated demand with limited stress on City services.

(c) The types, size and location of structures.

The existing CBRF and RCAC buildings are masonry and stucco. The CBRF buildings are all one-story. The 42-unit combined RCAC and CBRF building is 3 stories. The existing office wing is one story with lease-able lower-level office space. No exterior changes are proposed for any of the existing structures on Lot 2.

The proposed new buildings on Lot 1 and Lot 3 have not been designed. However, they will comply with the Chapter 5 Design Guidelines of the North Fitchburg Road Plan, which are included in this application by reference. Illustrative building footprints are shown on the General Development Plan.

(d) A general utility plan.

The existing development on Lot 2 is currently served by municipal water and sanitary sewer service. No change is proposed in the service to these properties.

The attached Utility Plan shows the proposed connections for new development on Lot 1 and Lot 3, which will be from utility lines in the adjoining public street rights-of-way.

(e) Recreational and Open Space Areas

The CBRF and RCAC facilities generate little demand for outdoor recreation facilities and open space. However there are three areas proposed for passive recreation:

- a. Interior Plaza: An interior landscaped plaza is currently in place between the 42-unit building and the adjoining CBRF structure. The plaza is a visual amenity and is accessible from several interior community spaces.
- b. Community Garden: A new community garden is proposed near the senior housing units (Lot 2)
- c. Apple Orchard: The existing apple orchard roughly one-acre in size will be deed restricted and protected. The apple orchard is part of Lot 3.

(f) General landscape treatment plan.

The General Landscape Plan is shown on the attached plan sheet. The landscape plan includes the following features:

- a. Preservation of existing mature vegetation along the north and south property lines
- b. Preservation of the apple orchard through deed restriction
- c. Replacement of removed trees on Lot 1 with new canopy tree plantings.
- d. Development of a community garden for the independent senior units
- e. Landscape improvement to the interior plaza.
- f. Maintenance of the stormwater management basin in the northeast corner of the property.

(g) Statistical data

The following are the proposed density / intensity standards, land use analysis, and dwelling unit analysis for the development

See following page for land use data:

LAND USE TABLE							
Avaion Senior Campus			***		Lot 3	Outlot 1	Total Lots
Lot #	<u></u>	בים ז	LO. 2	Loit B		-	
			Ī		Ī	30 253 ef 0 90 Acres	463.564 sf: 10.62 Acres
Lot Area		60,259 sf; 1.88 Acres		3.56 Acres	31, 2.00 ACI 85	١	301 317 ef
May Allowable Impervious Area 65%	Γ			į	80,839 sf		460 047 of
Min Open Space 35%	Т		24.276 sf		43,528 sf		102,247 SI
Actual Imposizione (Actual Open Space	7			82,910 sf (37%)	-		
Actual III per vicual Actual							
	Т	Darking	CBRF	CBRF/RCAC/Offices	Undeveloped	Undeveloped	
Ose	Proposod	reial	andent Senior Housing		Independent Senior Housing	Stormwater Mngmnt	
Descloyment Dhees	Т		Existing/ Future Conversion	Existing to Remain	Future	Develops with other lots	
Developinent i nase			Ţ				
Mary Englastics Area		7 350 sf			approx. 20,000 sf		
Max Footblan Area (1)		3.	18.513 sf	88,275 sf	80,000 sf (4 levels)		
Max Building Avea (1)					1.24		
Floor Area Naulo				3 CBRF	3 w/ underground parking		
Number of Storeys		(1) S (2) Z					
			Т	74 CBRF/RCAC			
Living Units	Existing		oz neg con		120 unit Ind Senior Housing		
	Proposed		1		42 1 inits/acre		
Density (units / acre)			10.6 units/acre				
Parking	Existing	46 surface		45 Surface			
0	Required	<u>(S)</u>	32 2/unit	60 (2)	180 1.5/unit		
				45 + 27new = 72 surface			
	Т	e 2 etalle from 2A	0	17	0; needs 4 stails from Lot 2B		
	Dalined						
Setbacks:					15.	₹Z	
Front Setback		15	15.		25	INA	
Sideyard Setback		15'	15	CI.	25	NA	
Rearyard Setback		10,	10,	.01	9		

Notes:

(1) Note. Max building area and number of storeys (to 3) for Lot 1 can increase with underground parking
(2) CBRF/RCAC parking = 1 stall/6 beds + 1 stall/employee + 1 stall/visiting physician; office 1 stall/300 sf
(2) CBRF/RCAC parking = 1 stall/6 beds + 13 employees + 4 Physicians + (9,000/300) = 13+13+4+30=60 stalls

Organizational Structure

The property will be owned by either three four ownership entities:

Parcel A - Unknown

Parcel B - Ridgeline Management Company

Parcel C - Avalon Senior Care (Letter of Intent to sell to Ridgeline Management Company)

Parcel D - Unknown

The stormwater management area (Outlot 1) will be co-owned by the owners of Parcel B and C and will be managed under a shared use agreement.

PLANNED DEVELOPMENT DISTRICT SPECIFIC IMPLEMENTATION PLAN (PDD - SIP)

1. INTRODUCTION

Avalon Senior Care is submitting an application for rezoning from the Residential – Low Density District (R-L) with a C.U.P for a 106-bed senior care facility to Planned Development District – General Implementation Plan (PDD-GIP) for the entire Homeville Fitchburg LLC (Avalon Senior Care) property at 2875 Fish Hatchery Road

A Specific Implementation Plans (SIP's) and Certified Survey Map (CSM) is concurrently submitted for the existing structures. Approval of the PDD-SIP and CSM will enable sale of the properties.

No exterior changes or site improvements are proposed for any of the existing structures subject to this SIP.

The rezoning to PDD-GIP and PDD-SIP rezoning has been requested by City staff, since the proposed parcels with the existing buildings will be served by a private road and will not have direct access on a public street.

2. PROJECT DESCRIPTION

Existing Conditions

The portion of the property subject to the proposed SIP consists of an existing Community Based Residential Facility (CBRF) and Residential Care Apartment Complex (RCAC) licensed for 106 beds. The beds are located in a 3-story 42-unit CBRF / RCAC structure and in three separate one-story CBRF structures, one of which is connected by an indoor passageway to the main 42-unit facility. In addition to the senior care units, there are approximately 9,000 square feet of office space in an office wing connected to the 42-unit building. The current office tenants are senior care related agencies.

Avalon Senior Car, which operates the existing facilities, currently has 34 full-time and 12 part-time employees. The maximum number of employees on shift at any given time is 13.

Proposed Development

The existing 42-unit building, connected office wing, and two of the CBRF buildings are proposed for sale to Ridgeline Management Company, a national organization based in Eugene, Oregon that specializes in the management and operation of assisted living, memory care (Alzheimer's care) and independent retirement communities.

The third existing CBRF building will be converted from a CBRF facility licensed for 32 beds to 16 independent senior housing units. Ridgeline management Company has submitted a Letter of Intent to purchase this portion of the property in the future.

See following page for land use chart.

LAND USE TABLE							
Avalon Senior Campus							- T
tot#		Lot 1	Lot 2		Lot 3	Outfot 1	otal Lots
				Unit B			
Lot Area		60,259 sf; 1.88 Acres	1.59 Acres	155,882 sf; 3.58 Acres	sf; 2.85 Acres	39,253 sf; 0.90 Acres	463,564 sf; 10.62 Acres
Max Allowable Impervious Area 65%	s Area 65%	37,811 sf			80,839 sf		301,317 sf
Min Open Space 35%		22,448 sf			43,528 sf		162,247 sf
Actual Impervious/Actual Open Space	Open Space		142,331 sf (63%);	82,910 sf (37%)			
Use	Existing	Parking	CBRF	CBRF/RCAC/Offices	Undeveloped	Undeveloped	
	Proposed	rcial	endent Senior Housing	CBRF/RCAC/Offices	ndent Senior Housing	Stormwater Mngmnt	
Development Phase		Future		Existing to Remain	Future	Develops with other lots	
Max Footprint Area		7,350 sf			approx. 20,000 sf		
Max Building Area (1)		J6	3.sf	88,275 sf	80,000 sf (4 levels)		
Floor Area Ratio			0.28		1.24		
Number of Storeys		2 to 3 (1)		1 Offices, 3 CBRF	3 w/ underground parking		
Living Units	Existing		32 bed CBRF	74 CBRF/RCAC			
	Proposed		16 unit Ind. Senior Housing	74 CBRF/RCAC	120 unit Ind. Senior Housing		
Density (units / acre)			10.6 units/acre	20.8 beds/acre	42.1 units/acre		
		,			,		
Parking	Existing	46 surface	0	45 Surface	0	0	
	Required	(3)	5/unit; 32 2/unit	60 (Z)	180 1.5/unit		
	Proposed	46 (3)	42 Surface	7new = 72 surface	56 Surface; 120 Underground		
	"Banked"	s 3 stalls from 2A	0	17	0; needs 4 stalls from Lot 2B		
Setbacks:				,			
Front Setback		15	15			NA	
Sideyard Setback		15	15'			NA	
Rearyard Setback		10'	10.	10'	10'	NA	

Notes:
(1) Note. Max building area and number of storeys (to 3) for Lot 1 can increase with underground parking
(2) CBRF/RCAC parking = 1 stall/6 beds + 1 stall/employee + 1 stall/visiting physician; office 1 stall/300 sf
(2) CBRF/RCAC parking = 1 stall/8 beds = 13 stalls + 13 employees + 4 Physicians + (9,000/300) = 13+13+4+30=60 stalls

PLANNED DEVELOPMENT DISTRICT GENERAL IMPLEMENTATION PLAN (PDD - GIP)

Appendix A: Contacts

Property Owner:

Homeville Fitchburg, LLC Contract Person: Bill Clemens 200 Meadow Oak Trail Waunakee, WI 53597 608-575-8642

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Surveyor:

Birrenkott Surveying, Inc.
Contact Person: Dan Birrenkott, RLS
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Sun Prairie, WI 53590
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birrenkott@spwl.net

Stormwater Engineering:

Quam Engineering, LLC Contact Person: Kevin J. Parish, P.E. 4893 Larson Beach Road McFarland, WI 53558 608 838-7750 kiparish@SBCGlobal.net

PLANNED DEVELOPMENT DISTRICT GENERAL IMPLEMENTATION PLAN (PDD - GIP)

Appendix B: Maps

- 2.1 General Development Plan
- 2.2 Conceptual Landscape and Open Space Plan
- 2.3 Current Conditions Plan
- 2.4 Parcel Map
- 2.5a Certified Survey Map (overall)
- 2.5b Certified Survey Map Detail
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- 2.6 Condominium Map
- 2.7 Utility Map
- 2.8 Topography Map
- 2.9 Tree Location Map
- 2.10 Existing Drainage Plan
- 2.11 Proposed Drainage Plan
- 2.12 Preliminary Stormwater Management Plan

PLANNED DEVELOPMENT DISTRICT GENERAL IMPLEMENTATION PLAN (PDD - GIP)

Appendix C: Preliminary Storm Water Management Calculations

PRELIMINARY STORM WATER MANAGEMENT CALCULATIONS

AVALON SENIOR CAMPUS CITY OF FITCHBURG, WISCONSIN

February 16, 2007

PREPARED FOR:

Bill Clemens 106 East Doty Street Madison, WI 53703

PREPARED BY:
Quam Engineering, LLC 4893 Larson Beach Road McFarland, WI 53558

BI-02-07

TABLE OF CONTENTS

Introduction	Page 1
Standards	Page 2
Storm Water Management	Page 3
Storm Water Management	Page 4
Results	Dage 5
Conclusions	rages

EXHIBITS

1.	Location Map
2.	Current Conditions Plan
3.	Existing Drainage Plan
4.	Proposed Drainage Plan
-	nutiminary Stormwoter Management Plan

APPENDICES

A.	Pre-Construction Hydrographs
B.	Post-Construction Hydrographs
C.	Pond and Outfall Structure Reports
D.	Pond Efficiency Calculations
E.	Infiltration and Storm Sewer Calculations
F.	Soils Information

INTRODUCTION

The proposed development is located at 2875 Fish Hatchery Road in the City of Fitchburg, Dane County. The site is part of the NE ¼ of the NE ¼ of Section 3, T6N, and R9E, as shown on the Location Map included as Exhibit # 1. The existing site consists of a senior living campus as shown on the Current Conditions Plan included as Exhibit #2. The proposed development will include adding a commercial development with parking along Fish Hatchery Road and an additional senior living building with parking in the southeast corner of the parcel.

The soils in this area consist of Elburn, McHenry, Plano and Ringwood soils, which are classified in Hydrologic Soil Group "B", as shown in Appendix E.

The intent of this report is to provide details on how the stormwater will be collected and managed so that it leaves the proposed development in accordance with the City of Fitchburg stormwater management standards.

STANDARDS

The stormwater management system for the proposed development will meet the following performance standards as defined in the Wisconsin Administrative Code NR 151 and Chapter 27 of the City of Fitchburg Standards:

Sediment Control

The proposed construction shall include design practices to retain soil particles greater than five microns (80% reduction) on the entire site resulting from the one-year 24-hour storm event

Oil and Grease Control

The first ½" runoff shall be treated using oil and grease removal technology.

Runoff Rate Control

All stormwater facilities shall be designed, installed and maintained to effectively maintain pre-development peak runoff rates for the 2, 10 and 100 year, 24-hour storm event. The Dane County rainfall values for all storm events are as follows:

Sylvanias in allega non-indular sens	Harry Martin Commission (All States and All States
Storm	Rainfall
Event	Intensity
(Year)	(in/hr)
2	2 .9
10	4.2
100	6.0

Outlets

Discharges from the development must have a stable outlet capable of carrying designed flow at a non-erosive velocity.

Infiltration

For nonresidential development, including commercial, industrial and institutional development, design practices to infiltrate sufficient runoff volume so that post-development infiltration volume shall be at least 60% of the pre-development infiltration volume, based on average annual rainfall. When designing appropriate infiltration systems, no more than two percent (2%) of the site is required to be used as effective infiltration area if the infiltration system has been designed to the maximum extent practicable.

Thermal Control

The stormwater management plan shall include provisions and practices to reduce the temperature of runoff for sites located within the watershed of a river or stream identified by the Wisconsin DNR as a cold water community.

STORMWATER MANAGEMENT MEASURES

Exhibit #5 is the Preliminary Stormwater Management Plan. The plan shows the stormwater management measures required to meet the standards listed on page 2 of this report. The standards will be met as follows:

Sediment Control

The detention pond will retain soil particles greater than five microns (80% reduction) on the entire site resulting from the one-year 24-hour storm event. The Pond Settling Efficiency Calculations are included in Appendix D.

Oil and Grease Control

Oil and grease runoff will be treated by the bio-retention devices.

Runoff Rate Control

The proposed detention pond will effectively maintain pre-development peak runoff rates for the 2, 10 and 100 year, 24-hour storm event. The results of the stormwater modeling are included on page 4 of this report.

Outlets

Riprap will be installed at all discharge points to minimize velocities and erosion associated with stormwater runoff.

Infiltration

The proposed non-residential development is required to meet Dane County and Wisconsin DNR NR151 standards requiring infiltration of 60% of the pre-development infiltration volume. The infiltration standard will be met using bio-retention devices located in the areas shown on Exhibit #5.

Thermal Control

Thermal Control is not applicable for the proposed development. The proposed development is not located within the watershed of a river or stream identified by the Wisconsin DNR as a cold water community.

RESULTS

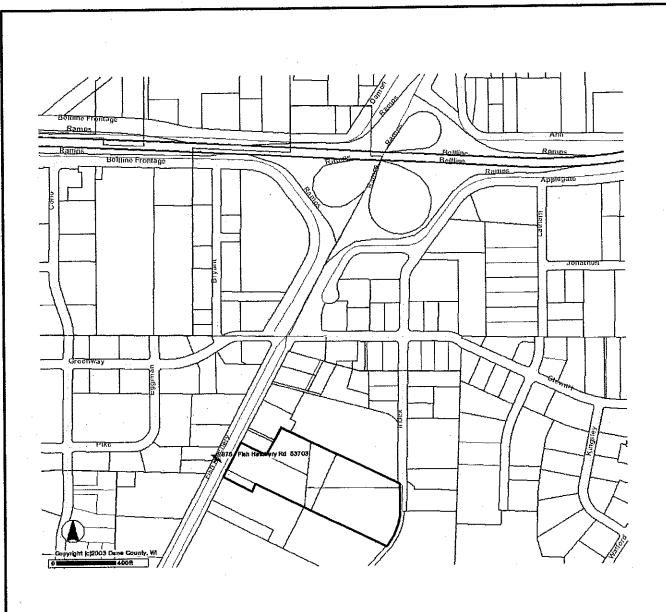
The following table summarizes the peak flow rates leaving the existing site with meadow land use, the proposed site without the pond and the proposed site with the pond for the various storm events. It also indicates the ratio of the proposed peak flow rate with the pond compared to the existing peak flow rate.

Storm Event (Year)	Existing Flow Rate (cfs)	Proposed Flow Rate Without Pond (cfs)	Proposed Flow Rate With Pond (cfs)	Proposed Versus Existing Flow Rate
2	1.42	20.85	0.74	0.52
10	8.33	38.70	4.61	0.55
100	23.87	66.91	19.03	0.80

CONCLUSIONS

As the results indicate, the storm water management measures for the proposed development meet the City of Fitchburg and Wisconsin Department of Natural Resources standards. The proposed detention pond will effectively maintain pre-development peak runoff rates for the 2, 10 and 100 year, 24-hour storm events. The Sediment Control Infiltration, Oil and Grease Control, Outlets and Thermal Control standards are achieved.

EXHIBITS



LOCATION MAP

EXHIBIT #1

APPENDIX A

PRE-CONSTRUCTION HYDROGRAPHS

IINTT HYDROGRAPH REPORT

Hydrograph Number:1 Name: Existing - Avalon C Type: SCS Curvilinear	ampus		
[UNIT HYDROGRAPH INFORMATION] Peak Flow (Qp) Time to Peak (Tp) Time of Base (Tb) Volume Shape Factor Time Step Excess Rain Storm Duration Lag Time			19.05 (cfs) 24.77 (min) 123.87 (min) 0.87 (ac-ft) 484.00 2.00 (min) 1.00 (in) 4.94 (min) 22.30 (min)
[BASIN INFORMATION] [WEIGHTED WATERSHED	D AREA]	·.	
Description	Area	CN	
Meadow	10.40	58	
Overall Approximation	10.40	58	
[TIME CONCENTRATION TR-55]	٠.		
SHEET FLOW Manning's Roughness Coef. (n) 2-yr 24-hr Rainfall (R) Flow Length (L) Land Slope (S) Travel Time of Sheet Flow		# # #	0.15 2.90 (in) 300.00 (ft) 0.03 (ft/ft) 20.21 (min)
SHALLOW FLOW K_Coefficient (K) Flow Length (L) Watercourse Slope (S) Travel Time of Shallow Flow Velocity (V)		= = =	0.50 500.00 (ft) 0.01 (ft/ft) 16.67 (min) 0.50 (ft/s)
CHANNEL FLOW Hydraulic Radius (R) Manning's Roughness Coef. (n) Flow Length (L) Channel Slope (S) Travel Time of Channel Flow Channel Velocity (V)		# # # #	1.00 (ft) 0.05 50.00 (ft) 0.01 (ft/ft) 0.28 (min) 2.98 (ft/s)
[TIME OF CONCENTRATION] Time of Concentration (Tc)		±	37.16 (min)

UNIT HYDROGRAPH REPORT

Hydrog	raph Number:2				
Name:	Existing - South				
Type:	SCS Curvilinear				
r Abe.					
F********	THE PACE THE THEODMARTON 1				
[UNIT	HYDROGRAPH INFORMATION]			=	28.26 (cfs)
	Peak Flow (Qp)			==	12.52 (min)
	Time to Peak (Tp)				
	Time of Base (Tb)			=	62.62 (min)
	Volume			==	0.65 (ac-ft)
	Shape Factor			=	484.00
				-	2.00 (min)
	Time Step				1.00 (in)
	Excess Rain				2,50 (min)
	Storm Duration				11.27 (min)
	Lag Time			=	II.2/ (MIII)
,					
[BASIN	INFORMATION]				
	[WEIGHTED WATERSHED	AREA]			
	Description	Area	CN		
	D000116 0101				
	Mondon	7.80	58		•
	Meadow				
		7.80	5.0		
	Overall Approximation	7.00	. 30		
[TIME	CONCENTRATION TR-55]				
	SHEET FLOW				
	Manning's Roughness Coef. (n)			=	0.15
	2-yr 24-hr Rainfall (R)			Ħ.	2.90 (in)
	Flow Length (L)			=	300.00 (ft)
				=	0.04 (ft/ft)
	Land Slope (S)			=	18.78 (min)
	Travel Time of Sheet Flow				200,0 (3,1,2,2)
	•				
	SHALLOW FLOW				2.05
	K Coefficient (K)		*	===	0.25
	Flow Length (L)			=	0.00 (ft)
	Watercourse Slope (S)	-		±	0.00 (ft/ft)
	Travel Time of Shallow Flow			=	0.00 (min)
				=	0.08 (ft/s)
	Velocity (V)				, , ,
	· · · · · · · · · · · · · · · · · · ·				
	CHANNEL FLOW			_	0.00 (ft)
	Hydraulic Radius (R)			=	• •
	Manning's Roughness Coef. (n)			=	0.01
	Flow Length (L)			=	0.00 (ft)
	Channel Slope (S)			=	0.00 (ft/ft)
	Travel Time of Channel Flow			=	0.00 (min)
				=	0.04 (ft/s)
	Channel Velocity (V)			-	0.00 (20/7/
[TIME	OF CONCENTRATION]				10 70 /
	Time of Concentration (Tc)			==	18.79 (min)

FLOOD HYDROGRAPH REPORT

```
Hydrograph Number:
                        1 Yr - Existing - Avalon Campus
Name:
                         Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                          0.27 (cfs)
      Peak Flow (Qp)
                                                          756.00 (min)
      Time to Peak (Tp)
                                                          1562.06 (min)
      Time of Base (Tb)
                                                          0.11 (ac-ft)
      Volume
Hydrograph Number:
                         2 Yr - Existing - Avalon Campus
Name:
                         Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                          0.80 (cfs)
      Peak Flow (Qp)
                                                          746.00 (min)
                                                    =
      Time to Peak (Tp)
                                                          1562.06 (min)
      Time of Base (Tb)
                                                          0.21 (ac-ft)
      Volume
Hydrograph Number:
                         10 Yr - Existing - Avalon Campus
Name:
                         Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                          4.37 (cfs)
      Peak Flow (Qp)
                                                          740.00 (min)
      Time to Peak (Tp)
                                                          1562.06 (min)
       Time of Base (Tb)
                                                          0.65 (ac-ft)
      Volume
Hydrograph Number:
                         100 Yr - Existing - Avalon Campus
Name:
                         Computed Flood
Type:
 [HYDROGRAPH INFORMATION]
                                                           12.31 (cfs)
       Peak Flow (Qp)
                                                           738.00 (min)
       Time to Peak (Tp)
                                                           1562.06 (min)
       Time of Base (Tb)
                                                           1.52 (ac-ft)
       Volume
Hydrograph Number:
                         1 Yr - Existing - South
Name:
                         Computed Flood
 Type:
 [HYDROGRAPH INFORMATION]
                                                           0.24 (cfs)
       Peak Flow (Qp)
                                                           734.00 (min)
       Time to Peak (Tp)
                                                           1501.70 (min)
       Time of Base (Tb)
                                                           0.08 (ac-ft)
       Volume
 Hydrograph Number:
                          2 Yr - Existing - South
 Name:
                          Computed Flood
 Type:
 [HYDROGRAPH INFORMATION]
                                                           0.89 (cfs)
       Peak Flow (Qp)
                                                           730.00 (min)
       Time to Peak (Tp)
                                                           1501.70 (min)
       Time of Base (Tb)
                                                           0.15 (ac-ft)
       Volume
```

Preliminary Storm Calculations

BI-02-07

FLOOD HYDROGRAPH REPORT

```
Hydrograph Number:
                        10 Yr - Existing - South
Name:
                        Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                          5.30 (cfs)
      Peak Flow (Qp)
                                                          726.00 (min)
      Time to Peak (Tp)
                                                          1501.70 (min)
      Time of Base (Tb)
                                                          0.49 (ac-ft)
      Volume
Hydrograph Number:
                         100 Yr - Existing - South
Name:
                         Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                          14.59 (cfs)
      Peak Flow (Qp)
                                                          726.00 (min)
      Time to Peak (Tp)
                                                          1501.70 (min)
      Time of Base (Tb)
                                                          1.14 (ac-ft)
      Volume
Hydrograph Number:
                         1 Yr - Existing - Total
Name:
                         Combined
Type:
[HYDROGRAPH INFORMATION]
                                                          0.49 (cfs)
      Peak Flow (Qp)
                                                          750.00 (min)
      Time to Peak (Tp)
                                                          1498.00 (min)
      Time of Base (Tb)
                                                          0.20 (ac-ft)
      Volume
                         10
Hydrograph Number:
                         2 Yr - Existing - Total
Name:
                         Combined
Type:
[HYDROGRAPH INFORMATION]
                                                          1.42 (cfs)
       Peak Flow (Qp)
                                                          734.00 (min)
       Time to Peak (Tp)
                                                          1504.00 (min)
       Time of Base (Tb)
                                                          0.36 (ac-ft)
       Volume
Hydrograph Number:
                         11
                         10 Yr - Existing - Total
Name:
                         Combined
Type:
 [HYDROGRAPH INFORMATION]
                                                          8.33 (cfs)
       Peak Flow (Qp)
                                                          730.00 (min)
       Time to Peak (Tp)
                                                          1516.00 (min)
       Time of Base (Tb)
                                                          1.15 (ac-ft)
       Volume
Hydrograph Number:
                         100 Yr - Existing - Total
Name:
                         Combined
Type:
 [HYDROGRAPH INFORMATION]
                                                          23.87 (cfs)
                                                    =
       Peak Flow (Qp)
                                                          728.00 (min)
                                                    =
       Time to Peak (Tp)
                                                          1526.00 (min)
                                                    ===
       Time of Base (Tb)
                                                          2.66 (ac-ft)
       Volume
```

Preliminary Storm Calculations

BI-02-07 2/16/2007

APPENDIX B

POST-CONSTRUCTION HYDROGRAPHS

UNIT HYDROGRAPH REPORT

Hydrog Name: Type:	raph Number:1 Proposed Developme SCS Curvilinear	ent			
	HYDROGRAPH INFORMATION] Peak Flow (Qp) Time to Peak (Tp) Time of Base (Tb) Volume Shape Factor Time Step Excess Rain Storm Duration Lag Time				36.13 (cfs) 13.06 (min) 65.30 (min) 0.87 (ac-ft) 484.00 2.00 (min) 1.00 (in) 2.61 (min) 11.75 (min)
[BASIN	INFORMATION]				
	[WEIGHTED WATERSH	ed area; 		.	
	Description	Area	CN		
	Impervious Lawn - HSG C	6.03 4.37			· .
•	Overall Approximation	10.40	88		
[TIME	CONCENTRATION TR-55]				
	SHEET FLOW Manning's Roughness Coef. (n) 2-yr 24-hr Rainfall (R) Flow Length (L) Land Slope (S) Travel Time of Sheet Flow			= = = =	0.15 2.90 (in) 125.00 (ft) 0.01 (ft/ft) 16.23 (min)
	SHALLOW FLOW K_Coefficient (K) Flow Length (L) Watercourse Slope (S) Travel Time of Shallow Flow Velocity (V)			= = =	0.25 0.00 (ft) 0.00 (ft/ft) 0.00 (min) 0.08 (ft/s)
	CHANNEL FLOW Hydraulic Radius (R) Manning's Roughness Coef. (n) Flow Length (L) Channel Slope (S) Travel Time of Channel Flow Channel Velocity (V)				0.50 (ft) 0.01 860.00 (ft) 0.00 (ft/ft) 3.36 (min) 4.27 (ft/s)
(TIME	OF CONCENTRATION] Time of Concentration (Tc)			*	19.59 (min)
Prelimii BI-02-0	nary Storm Calculations				
2/16/20		B-1		•	

UNIT HYDROGRAPH REPORT

Hydrog Name: Type:	raph Number:2 Existing - South SCS Curvilinear				
	HYDROGRAPH INFORMATION] Peak Flow (Qp) Time to Peak (Tp) Time of Base (Tb) Volume Shape Factor Time Step Excess Rain Storm Duration Lag Time			=======================================	28.26 (cfs) 12.52 (min) 62.62 (min) 0.65 (ac-ft) 484.00 2.00 (min) 1.00 (in) 2.50 (min) 11.27 (min)
[BASIN	I INFORMATION] [WEIGHTED WATERSHED	AREA]			
	Description	Area	CN		
	Meadow	7.80	58		
	Overall Approximation	7.80	58		•
[TIME	CONCENTRATION TR-55]				
	SHEET FLOW Manning's Roughness Coef. (n) 2-yr 24-hr Rainfall (R) Flow Length (L) Land Slope (S) Travel Time of Sheet Flow		·	= = = =	0.15 2.90 (in) 300.00 (ft) 0.04 (ft/ft) 18.78 (min)
	SHALLOW FLOW K_Coefficient (K) Flow Length (L) Watercourse Slope (S) Travel Time of Shallow Flow Velocity (V)			# # # # # # # # # # # # # # # # # # #	0.25 0.00 (ft) 0.00 (ft/ft) 0.00 (min) 0.08 (ft/s)
	CHANNEL FLOW Hydraulic Radius (R) Manning's Roughness Coef. (n) Flow Length (L) Channel Slope (S) Travel Time of Channel Flow Channel Velocity (V)		·	# # # # #	0.00 (ft) 0.01 0.00 (ft) 0.00 (ft/ft) 0.00 (min) 0.04 (ft/s)
[TIME	OF CONCENTRATION] Time of Concentration (Tc)			=	18.79 (min)

Preliminary Storm Calculations BI-02-07 2/16/2007

FLOOD HYDROGRAPH REPORT

```
Hydrograph Number:
                         1 Yr - Proposed Development
Name:
                         Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                          16.11 (cfs)
      Peak Flow (Qp)
                                                          724.00 (min)
      Time to Peak (Tp)
                                                          1503.57 (min)
                                                    =
      Time of Base (Tb)
                                                          1.20 (ac-ft)
      Volume
Hydrograph Number:
                         2 Yr - Proposed Development
Name:
                         Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                          20.16 (cfs)
      Peak Flow (Qp)
                                                          724.00 (min)
      Time to Peak (Tp)
                                                          1503.57 (min)
      Time of Base (Tb)
                                                          1.50 (ac-ft)
      Volume
Hydrograph Number:
                         10 Yr - Proposed Development
Name:
                         Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                           33.64 (cfs)
       Peak Flow (Qp)
                                                           724.00 (min)
       Time to Peak (Tp)
                                                           1503.57 (min)
       Time of Base (Tb)
                                                           2.53 (ac-ft)
       Volume
Hydrograph Number:
                         100 Yr - Proposed Development
Name:
                         Computed Flood
Type:
 [HYDROGRAPH INFORMATION]
                                                           52.42 (cfs)
       Peak Flow (Qp)
                                                           724.00 (min)
       Time to Peak (Tp)
                                                           1503.57 (min)
       Time of Base (Tb)
                                                           4.01 (ac-ft)
       Volume
Hydrograph Number:
                          1 Yr - Existing - South
Name:
                          Computed Flood
 Type:
 [HYDROGRAPH INFORMATION]
                                                           0.24 (cfs)
       Peak Flow (Qp)
                                                           734.00 (min)
       Time to Peak (Tp)
                                                           1501.70 (min)
       Time of Base (Tb)
                                                           0.08 (ac-ft)
       Volume
 Hydrograph Number:
                          2 Yr - Existing - South
 Name:
                          Computed Flood
 Type:
 [HYDROGRAPH INFORMATION]
                                                           0.89 (cfs)
       Peak Flow (Qp)
                                                           730.00 (min)
       Time to Peak (Tp)
                                                           1501.70 (min)
       Time of Base (Tb)
                                                           0.15 (ac-ft)
       Volume
```

Preliminary Storm Calculations

BI-02-07

FLOOD HYDROGRAPH REPORT

```
Hydrograph Number:
                        10 Yr - Existing - South
Name:
                        Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                         5.30 (cfs)
      Peak Flow (Qp)
                                                         726.00 (min)
                                                   =
      Time to Peak (Tp)
                                                         1501.70 (min)
                                                   ==
      Time of Base (Tb)
                                                         0.49 (ac-ft)
      Volume
Hydrograph Number:
                        100 Yr - Existing - South
Name:
                        Computed Flood
Type:
[HYDROGRAPH INFORMATION]
                                                         14.59 (cfs)
      Peak Flow (Qp)
                                                         726.00 (min)
      Time to Peak (Tp)
                                                         1501.70 (min)
      Time of Base (Tb)
                                                         1.14 (ac-ft)
      Volume
Hydrograph Number:
                        1 Yr - Proposed - Total
Name:
                        Combined
Type:
[HYDROGRAPH INFORMATION]
                                                         16.23 (cfs)
      Peak Flow (Qp)
                                                         724.00 (min)
      Time to Peak (Tp)
                                                         1480.00 (min)
      Time of Base (Tb)
                                                          1.28 (ac-ft)
      Volume
                        10
Hydrograph Number:
                         2 Yr - Proposed - Total
Name:
                         Combined
Type:
[HYDROGRAPH INFORMATION]
                                                         20.85 (cfs)
      Peak Flow (Qp)
                                                   = 724.00 (min)
      Time to Peak (Tp)
                                                         1482.00 (min)
      Time of Base (Tb)
                                                          1.65 (ac-ft)
      Volume
Hydrograph Number:
                         11
                         10 Yr - Proposed - Total
Name:
                         Combined
Type:
 [HYDROGRAPH INFORMATION]
                                                          38.70 (cfs)
       Peak Flow (Qp)
                                                          724.00 (min)
       Time to Peak (Tp)
                                                          1484.00 (min)
       Time of Base (Tb)
                                                          3.02 (ac-ft)
       Volume
                         12
 Hydrograph Number:
                         100 Yr - Proposed - Total
 Name:
                         Combined
 Type:
 [HYDROGRAPH INFORMATION]
                                                          66.91 (cfs)
       Peak Flow (Qp)
                                                          724.00 (min)
       Time to Peak (Tp)
                                                          1488.00 (min)
       Time of Base (Tb)
                                                          5.15 (ac-ft)
       Volume
 Preliminary Storm Calculations
 BI-02-07
                                    B-4
```

2/16/2007

FLOOD HYDROGRAPH REPORT

```
Hydrograph Number:
                        1 Yr - Routed - Proposed Development
Name:
                        Reservoir: Modified Puls
Type:
[HYDROGRAPH INFORMATION]
                                                      0.68 (cfs)
      Peak Flow (Qp)
                                                        938.00 (min)
      Time to Peak (Tp)
                                                         2880.00 (min)
      Time of Base (Tb)
                                                        1.28 (ac-ft)
      Volume
                                                         2.00 (min)
      Time Step
                                                         896.15 (ft)
      Peak Elevation
Hydrograph Number:
                        2 Yr - Routed - Proposed Development
Name:
                        Reservoir: Modified Puls
Type:
[HYDROGRAPH INFORMATION]
                                                         0.74 (cfs)
      Peak Flow (Qp)
                                                         974.00 (min)
      Time to Peak (Tp)
                                                         2880.00 (min)
      Time of Base (Tb)
                                                         1.65 (ac-ft)
      Volume
                                                         2.00 (min)
      Time Step
                                                         896.62 (ft)
      Peak Elevation
Hydrograph Number:
                        10 Yr - Routed - Proposed Development
Name:
                        Reservoir: Modified Puls
Type:
 [HYDROGRAPH INFORMATION]
                                                         4.61 (cfs)
      Peak Flow (Qp)
                                                         770.00 (min)
      Time to Peak (Tp)
                                                         2880.00 (min)
      Time of Base (Tb)
                                                      3.00 (ac-ft)
      Volume
                                                         2.00 (min)
      Time Step
                                                         897.46 (ft)
      Peak Elevation
                        16
Hydrograph Number:
                        100 Yr - Routed - Proposed Development
Name:
                        Reservoir: Modified Puls
Type:
 [HYDROGRAPH INFORMATION]
                                                          19.03 (cfs)
       Peak Flow (Qp)
                                                          746.00 (min)
       Time to Peak (Tp)
                                                          2880.00 (min)
       Time of Base (Tb)
                                                         5.12 (ac-ft)
       Volume
                                                         2.00 (min)
       Time Step
                                                         898.59 (ft)
```

Peak Elevation

APPENDIX C

POND AND OUTFALL STRUCTURE REPORTS

RESERVOIR REPORT

Reservoir Number: 1

Name: Proposed Detention Pond

[RATING CURVE LIMIT]

Minimum Elevation = 893.00 (ft)
Maximum Elevation = 899.00 (ft)
Elevation Increment = 0.50 (ft)

[STAGE STORAGE INFORMATION]

Storage Method: User-Defined Storage Input Method: Area

Number	Elevation (ft)	Area (sq ft)	Ave Area (sq ft)	Volume (cu ft)	Cumulative Volume (cu ft)
	z == == == == == == = = = = = = = = = =		A AA	0.00	0.00
1	893.00	0.00	0.00	0.20	0.20
2	893.01	40.00	20.00	•	2116.33
3	894.00	4235.00	2137.50	2116.13	
4	895.00	13475.00	8855.00	8855.00	10971.33
-	896.00	23112.00	18293.50	18293.50	29264.83
5	• •	26937.00	25024.50	25024.50	54289.33
6	897.00		28899.50	28899.50	83188.83
7	898.00	30862.00			116444.83
. 8	899.00	35650.00	33256.00	33256.00	110444.03

[DISCHARGE INFORMATION]

Structure Number: 1

Type: Name: Trapezoidal Weir

Existing Overflow

Structure Number: 2

Type:

Name:

Proposed Stand Pipe

[RESERVOIR STAGE STORAGE/DISCHARGE]

Elevation (ft)	Stage (ft)	Area (sq ft)	Storage (cu ft)	Discharge (cfs)	
	0.00	0.00	0.00	0.00	
893.00	0.50	2116.31	529.08	0.12	
393.50	1.00	4235.00	2116.91	0.31	
394.00	1.50	8855.00	5389.41	0.43	
394.50	2.00	13475.00	10971.91	0.52	
95.00	2.50	18293.50	18914.03	0.59	
95.50	3.00	23112.00	29265.41	0.66	
396.00	3.50	25024.50	41299.53	0.72	
396.50	4.00	26937.00	54289.91	1.28	
397.00	4.50	28899.50	68249.03	4.80	
397.50	5.00	30862.00	83189.41	6.88	
398.00	5.50	33256.00	99218.91	15.30	
398.50 399.00	6.00	35650.00	116445.41	110.02	
	· · · · · · · · · · · · · · · · · · ·	.	116445.41 (cu	ft)	

Maximum Storage = 116445.41 (cu ft Maximum Discharge = 110.02 (cfs)

Preliminary Storm Calculations

BI-02-07

2/16/2007

C-1

OUTLET STRUCTURE REPORT

tructure Number	: 3 : Stand Pipe			
ype	: Proposed Stand	Pipe		
ame RATING CURVE LIMIT]	. Itopopou a resident	- -		
Minimum Elevation		==	893.00	(ft)
Maximum Elevation		==	898.70	(ft)
Elevation Increment	t.	=	0.50	(ft)
STAND PIPE INFORMATION]			************	
[ORIFICE INFORMATION]		•	. 4.00	(ft)
Diameter		=	4.00 12.57	(ft)
Crest Length	. •	=	12.57	(ft)
Effective Crest Le		=	0.60	120/
Orifice Coefficien		=	1.00	
Fractional Open Ar	ea	-	1.00	
[WEIR INFORMATION]		==	898.20	(ft)
Crest Elevation		=	3.33	(10)
Weir Coefficient Exponential		=	1.50 "	
OPT 	IONAL ORIFICE INF	ORMATION]	. .	****
Structure Number	: 1 : Circular Orifi	ce		
Type			*	
[OPTIONAL ORIFICE INFORM Diameter		==	0.33	(ft)
Invert Elevation		=	893.25	(ft)
Orifice Coefficien	it	=	0.60	
Number of Openings		= .	1	
	: 2		1	
Number of Openings Structure Number Type	: 2 : Circular Orifi	= .ce	1	
Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM	: 2 : Circular Orifi			/ f + \
Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter	: 2 : Circular Orifi	=	0.58	(ft) (ft)
Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter Invert Elevation	: 2 : Circular Orifi MATION]		0.58 896.82	(ft) (ft)
Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter Invert Elevation Orifice Coefficien	: 2 : Circular Orifi MATION]	=	0.58 896.82 0.60	·
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Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter Invert Elevation Orifice Coefficien Number of Openings [CULVERT INFORMATION] Type [OUTLET STRUCTURE INFORM	: 2 : Circular Orifi at : : : : : : : : : : : : : : : : : :	# # #	0.58 896.82 0.60 5	(ft)
Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter Invert Elevation Orifice Coefficien Number of Openings [CULVERT INFORMATION] Type [OUTLET STRUCTURE INFORM Diameter	: 2 : Circular Orifi at : : : : : : : : : : : : : : : : : :	= = = rete - Gro	0.58 896.82 0.60 5 ove End with F	(ft)
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Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter Invert Elevation Orifice Coefficien Number of Openings [CULVERT INFORMATION] ====================================	: 2 : Circular Orifi at : : : : : : : : : : : : : : : : : :	= = = rete - Gro	0.58 896.82 0.60 5 ove End with F	(ft) Headwall (in) (ft)
Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter Invert Elevation Orifice Coefficien Number of Openings [CULVERT INFORMATION] Type [OUTLET STRUCTURE INFORM Diameter Invert Elevation Pipe Length Slope	: 2 : Circular Orifi at : : : : : : : : : : : : : : : : : :	= = = rete - Gro	0.58 896.82 0.60 5 ove End with F 24.00 893.00 460.00	(ft) Headwall (in) (ft)
Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter Invert Elevation Orifice Coefficien Number of Openings [CULVERT INFORMATION] Type [OUTLET STRUCTURE INFORM Diameter Invert Elevation Pipe Length Slope Manning's n Value	: 2 : Circular Orifi at : : : Circular Conc	rete - Gro	0.58 896.82 0.60 5 ove End with F 24.00 893.00 460.00 0.00	(ft) Headwall (in) (ft)
Number of Openings Structure Number Type [OPTIONAL ORIFICE INFORM Diameter Invert Elevation Orifice Coefficien Number of Openings [CULVERT INFORMATION] Type [OUTLET STRUCTURE INFORM Diameter Invert Elevation Pipe Length Slope	: 2 : Circular Orifi At : Circular Conc. (ATION)	rete - Gro	0.58 896.82 0.60 5 ove End with F 24.00 893.00 460.00 0.00 0.01	(ft) Headwall (in) (ft)

Preliminary Storm Calculations BI-02-07

	****	****		=========		Total
Elevation (ft)	Stage (ft)	Weirs (cfs)	Orifices (cfs)	Stand Pipe (cfs)	Culvert (cfs)	(cfs)
893.00 893.50 894.00 894.50 895.00 895.50 896.00 896.50 897.00	0.00 0.50 1.00 1.50 2.00 2.50 3.00 3.50 4.00	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	0.00 0.12 0.31 0.43 0.52 0.59 0.66 0.72 1.28	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	0.00 1.17 4.25 8.44 13.06 14.40 15.74 17.08 18.42	0.00 0.12 0.31 0.43 0.52 0.59 0.66 0.72 1.28
897.00 897.50 898.00 898.50 898.70	4.50 5.00 5.50 5.70	0.00 0.00 0.00 0.00	4.80 6.88 8.42 8.96	0.00 0.00 6.88 14.80	19.48 20.50 21.51 21.92	4.80 6.88 15.30 21.92

[ORIFICE STAGE VS. DISCHARGE]

Elevation (ft)	Stage (ft)	Orifice 1 (cfs)	Orifice 2 (cfs)	Orifice 3 (cfs)	Orifice 4 (cfs)	Total (cfs)
893.00 893.50 894.00 894.50 895.00 895.50 896.00 896.50 897.00 897.50 898.00	0.00 0.50 1.00 1.50 2.00 2.50 3.00 3.50 4.00 4.50 5.00	0.00 0.12 0.31 0.43 0.52 0.59 0.66 0.72 0.78 0.83 0.88 0.93	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	0.00 0.12 0.31 0.43 0.52 0.59 0.66 0.72 1.28 4.80 6.88 8.42
898.70	5.70	0.95	8.02	0.00	0.00	8.96

Structure Number	: 1			
Туре	: Trapezoidal Weir			
Name	: Existing Overflow			
[RATING CURVE LIMIT]			000 70	· (#+\
Minimum Elevation		=	898.70	(ft)
Maximum Elevation		==	899.00	(ft)
Elevation Increment	t	=	0.10	(ft)
[OUTLET STRUCTURE INFORM	ATIONI			
•	·-···	==	159.00	(ft)
Crest Length Angle of Walls		=	150.0000	
Crest Elevation	•	=	898.70	(ft)
Weir Coefficient		==	3.33	
Exponential		=	1.50	
[MAXIMUM DISCHARGE]			07 40	(cfs)
Q		=	87.49	(CTS)

Preliminary Storm Calculations BI-02-07

2/16/2007

APPENDIX D

POND SETTLING EFFICIENCY CALCULATIONS

POND SETTLING EFFICIENCY CALCULATIONS

The peak volume, peak discharge, and peak elevation are based on a 1-year storm.

Settling Time

= (Peak Volume) / (Peak Discharge)

 $= 32,875 \text{ ft}^3 / 0.68 \text{ cfs}$ = 48,346seconds

Settling Distance

= Peak Elevation - Outfall Invert

= 896.15 feet - 893.25 feet

= 2.90 feet

Settling Velocity

= (Settling Distance) / (Settling Time)

= 2.90 feet / 48,346 seconds = 0.000060 feet / second

The pond settling efficiency calculations have been based upon anticipated construction conditions.

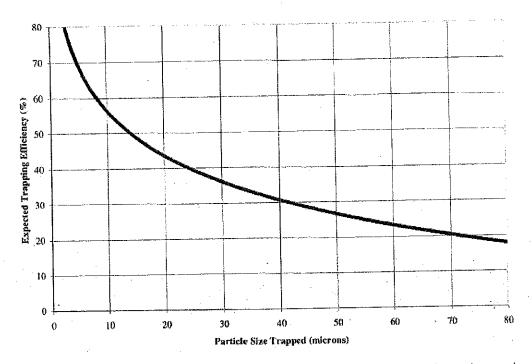
The attached chart indicates that according to Stokes Law with a specific gravity of 2.65 and 20° C, a $5\mu m$ diameter particle will settle to the pond bottom at this velocity. The attached graph shows that a pond that traps a $5\mu m$ particle is expected to be 80% efficient.

Settling Velocities for Spherical Particles - Stokes Law

Diame	ter	Velocity	Example Settling Time
(micr	1	(ft/s)	(hours for 2 foot depth)
	1	0.000003	185
CLAY	1.5	0.000007	76
	2	0.000012	46
	3	0.000026	21
	4	0.000045	12
	5	0.000073	8
	6	0.000106	5
	7	0.000138	4
	8	0.00019	3
SILT	9	0.00023	2.4
SILI	10	0.00029	2.
	12	0.00042	1.3
	15	0.00066	0.8
	20	0.0012	0.5
	25	0.0018	
	30	0.0027	0.2
	40	0.0047	0.1
	50	0.0074	0.07
SAND	60	0.011	0.05
JANU	80	0.019	0.03
	100	0.029	0.02

Note: Assumes specific gravity of 2.65 for soil particles and 20 degrees C water temperature.

DANE COUNTY EROSION CONTROL AND STORMWATER MANAGEMENT MANUAL



Convert the storage volume from the 1-year, 24-hour storm event into cubic feet. This volume of storage is then divided by the time required to settle the particle obtained by Stokes Law.

$$Q_{\max_{i,mum}}(cfs) = \frac{V_{Storage}(ft^3)}{Time(\sec)}$$

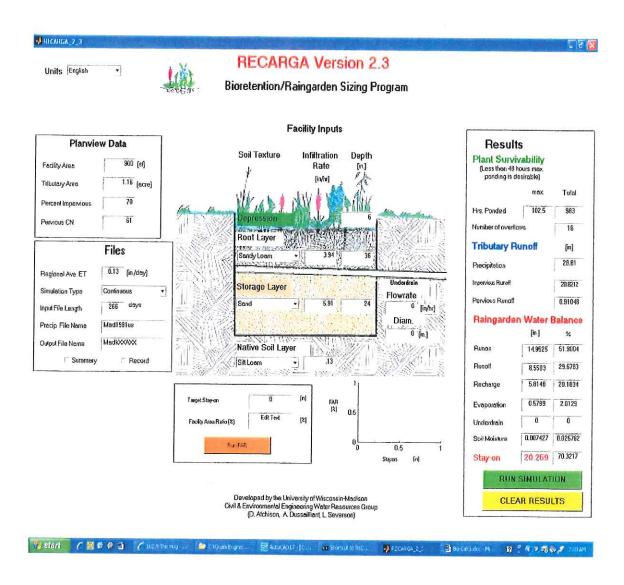
 $Q_{maximum}$ is the rate at which the basin must be released in order to obtain the expected efficiency.

*See table on following page for particle settling velocities to calculate Time (sec)

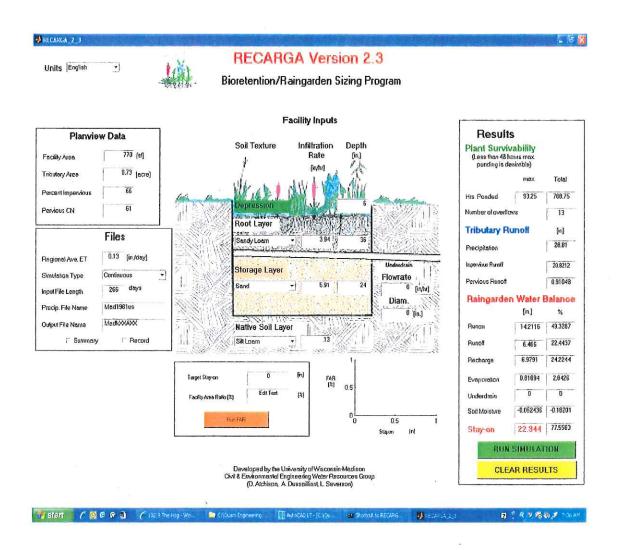
APPENDIX E

INFILTRATION AND STORM SEWER CALCULATIONS

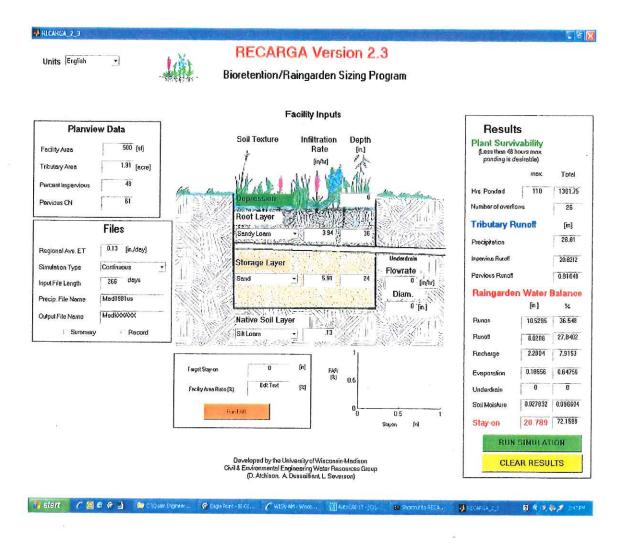
WEST BIO-RETENTION CALCULATIONS



NORTHWEST BIO-RETENTION CALCULATIONS



EAST BIO-RETENTION CALCULATIONS



WORK SHEET FOR STORM SEWER DESIGN

Avalon Senior Campus 2/16/2007 PROJECT: DATE:

Computed by: Checked by:

Kevin Parish

	tapacity	I 75	9.5	12.0	12.0	40.0	40.0	40.0	10.0	40.0	5.5	5.5						
SEWER	adole	Ft/Ft	0.0080	0.0130	0.0130	0.0035	0.0035	0.0035	0.0100	0.0035	0.0070	0.007				5 Figure 4.		
	9ziS 19w9S	<u> </u>	18	18	18	36	36	36	18	36	15	15				dure 13-10-	Chart 2.	M
	honuA ngisəO	CFS	8.46	12.03	12.03	14.60	30.86	30.86	8.39	39.25	5.32	5.32				-DM) Proce	10 Figure 4.	
- RUNOFF	Other Runoff	CFS	0.00	8.46	12.03	00.0	26.63	30.86	0.00	39.25	0.00	5.32				int Manual (F	dure 13-10-1	
RAINFALL - R	ThornA toerid	Q=C*I*A CFS	8.46	3.57	0.00	14.60	4.23	00.0	8.39	00.0	5.32	0.00	-	-10-5 Fig 2.	10-5 Fig 2.	consin (1905-1951) from Facilities Development Manual (FDM) Procedure 13-10-5 Figure	etermined from the nomograph in FDM Procedure 13-10-10 Figure 4, Chart 2	
3	Rain Intensity	(1) In./Hr.	8.1	8.1	1	1	8.1	,	6.1	1	8.1	1		FDM 13	FDM 13-	Facilities	ograph in	
	mot& ngis90	Yr.	100	100	100	100	100	100	10	100	100	100		-6%, from	6%, from	1951) from	n the nomo	- The state of the
BASIN	вэтА	(A)	1.16	0.49	1	-	0.58	-	1.91	1	0.73	1		C, Slope 2	, Slope 2-	in (1905-	nined fron	
8/	Runoff Coeff.	;(c)	06.0	0.90	ì	1	0.00	1	0.72	F	06.0	-1		il Group	i Group C	, Wiscons	vas deteri	
LOCATION	Downstream Structure		Inlet #6	CB #5	Ex. CB #3	Ex. CB #3	Ex. CB #2	CB #10	36" Elliptical	Ex. Outlet #1	CB #8	Outfall		* C=0.721; Commercial, Hydr. Soil Group C, Slope 2-6%, from FDM 13-10-5 Fig 2	C=0.90; Commercial, Hydr. Soil Group C, Slope 2-6%, from FDM 13-10-5 Fig 2.	= rainfall intensity for Madison, Wis	* Capacity Flowing Full for RCP was d	
COC	msentaqU enutounte		CB #7	Inlet #6	CB #5	Ex. End #4	Ex. CB #3	Ex. CB #2	CB #10	36" Elliptical	CB #8	CB #8		* C=0.721; Con	C=0.90; Com	= rainfall inte	* Capacity Flow	

APPENDIX F

SOILS INFORMATION

Feet 400

300

200

0 50 100

HYDROLOGIC GROUP RATING FOR DANE COUNTY, WISCONSIN





Web Soil Survey 1.1 National Cooperative Soil Survey Meters 100 20 25

HYDROLOGIC GROUP RATING FOR DANE COUNTY, WISCONSIN

MAP LEGEND

Hydrologic Group

[Dominant Condition, &It;]

Not rated or not available

Soil Map Units

O Cities

• Cities

Detailed Counties

Detailed States
Interstate Highways

Roads

Hydrography

Water

Oceans

MAP INFORMATION

Source of Map: Natural Resources Conservation Service Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov

Coordinate System: UTM Zone 16

Soil Survey Area: Dane County, Wisconsin Spatial Version of Data: 2 Soil Map Compilation Scale: 1:15840 Map comprised of aerial images photographed on these dates: 2000

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Tables - Hydrologic Group

Summary by Map Unit - Dane County, Wisconsin

Soil Survey Area Map Unit Symbol	Map Unit Name	Rating	Total Acres in AOI	Percent of AOI
EfB	Elburn silt loam, 1 to 4 percent slopes	В	2.0	9.5
MdC2	McHenry silt loam, 6 to 12 percent slopes, eroded	В	3.4	16.3
PnB	Plano silt loam, 2 to 6 percent slopes	В	2.8	13.4
RnB	Ringwood silt loam, 2 to 6 percent slopes	В	12.6	60.8

Description - Hydrologic Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are placed into four groups A, B, C, and D, and three dual classes, A/D, B/D, and C/D. Definitions of the classes are as follows:

The four hydrologic soil groups are:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only soils that are rated D in their natural condition are assigned to dual classes.

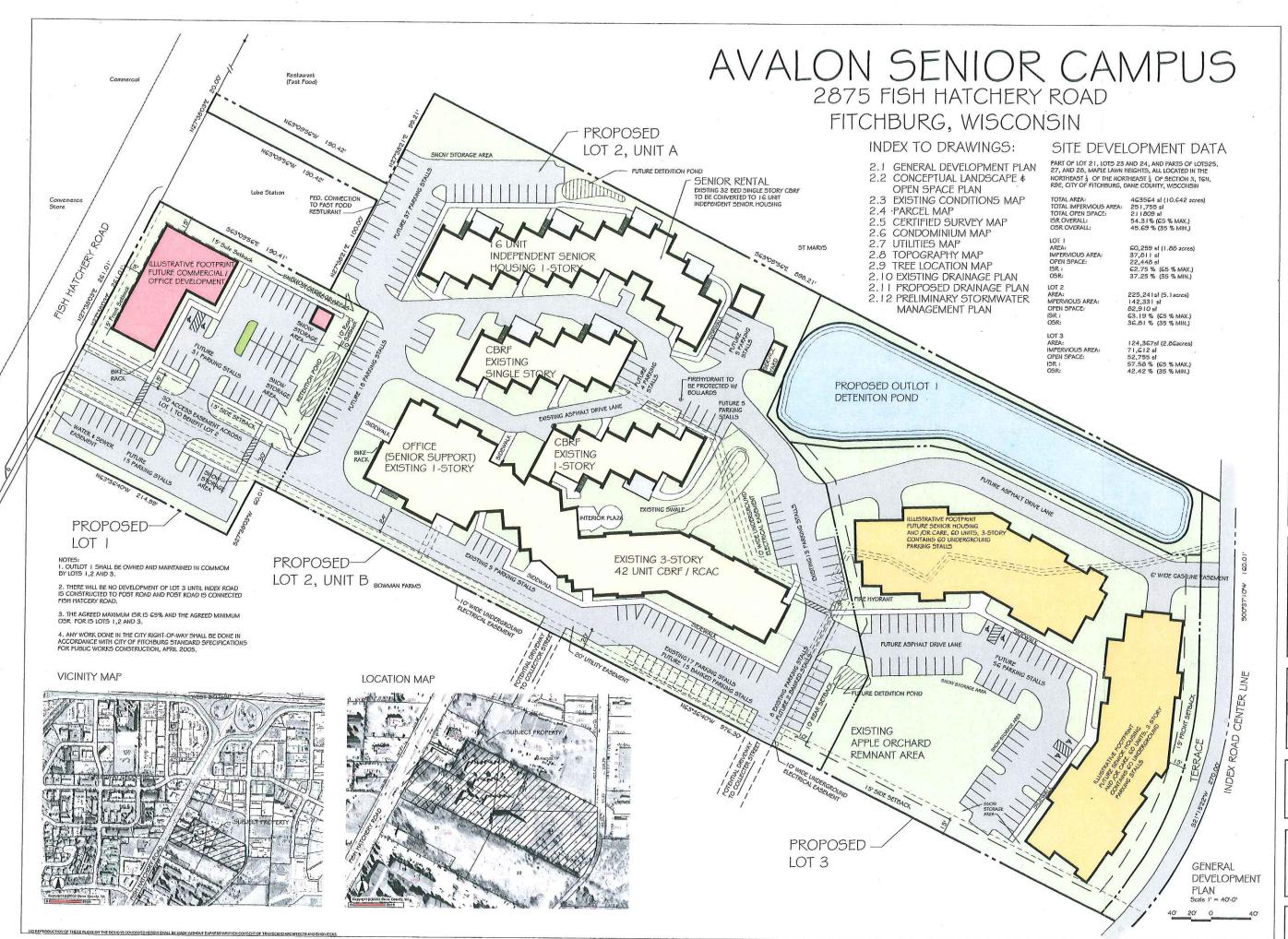
Parameter Summary - Hydrologic Group

Aggregation Method: Dominant Condition

Component Percent Cutoff:

Tie-break Rule: Lower

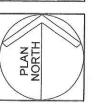






BIRRENKOTT SURVEYING INC. LAND SURVEYING &



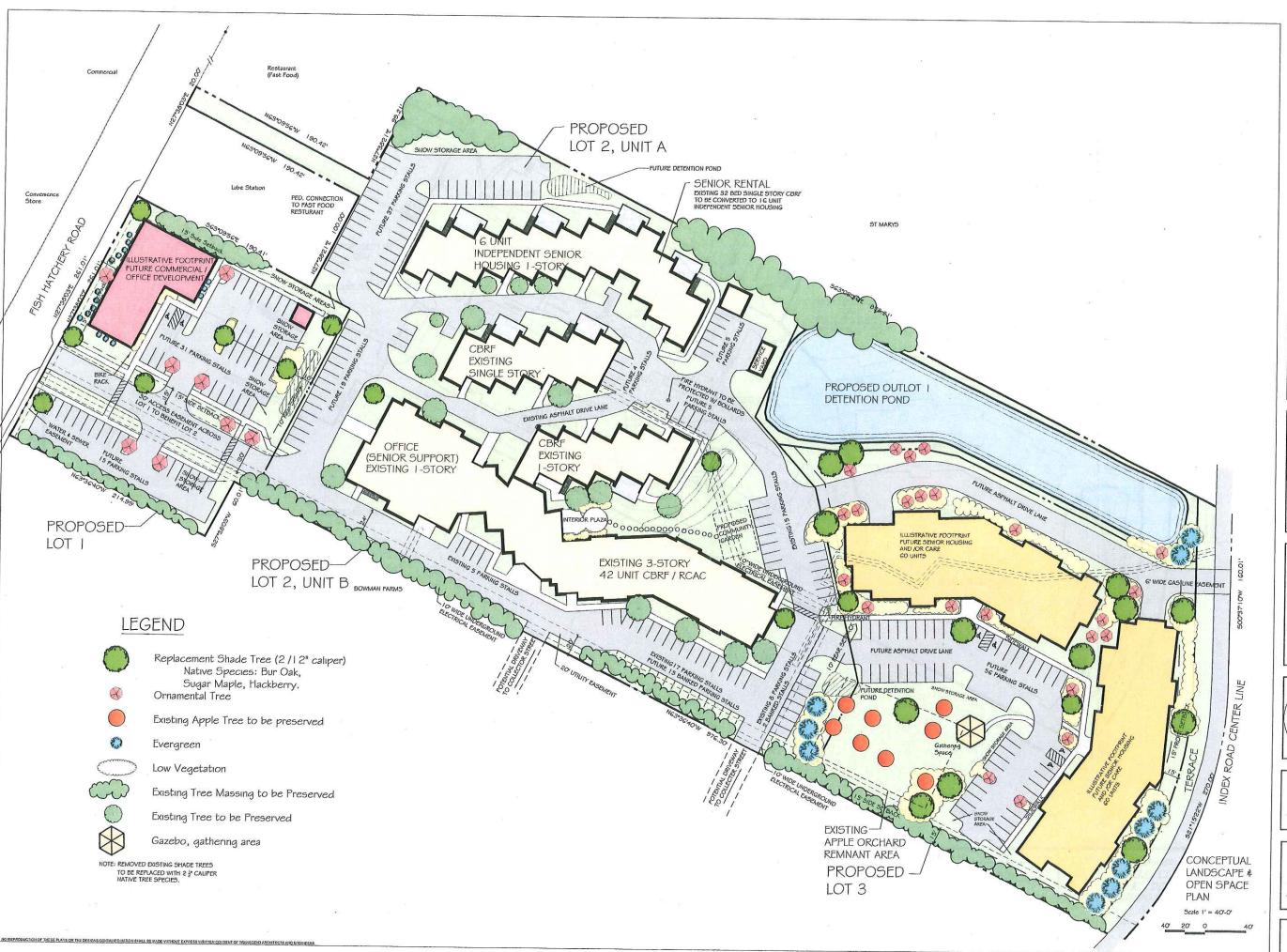


GENERAL DEVELOPMENT
PLAN

Scale 1' - 40'-0'

REVISIONS:

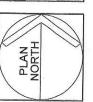
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CONCEPTUAL LANDSCAPE \$ OPEN SPACE PLAN

Scale 1* - 40-0'

REVISIONS:

P.O. Box 237 1677 N. Bristol Street Sun Prairie, WI. 53590 Phone (608) 837–7463 Fax (608) 837–1081 BIRRENKOTT SURVEYING, INC.

CERTIFIED SURVEY MAP

BEARINGS REFERENCED TO THE NORTH N89°40'54"W. PART OF LOT 21, LOTS 23 AND 24, AND PARTS OF LOTS 25, 27, AND 28, MAPLE LAWN HEIGHTS, ALL LOCATED IN THE NORTHEAST 1/4 OF THE NORTHEAST 1/4 OF SECTION 3, T6N, R9E, CITY OF FITCHBURG, DANE COUNTY, WISCONSIN

Legend:

• = FOUND IRON STAKE

O = SET 1"x24" IRON PIPEMIN. WGHT. 1.13 LBS/FT

Prepared For:
Avalon Senior Care
Bill Clemons
200 Meadow Oak Trail
Waunakee, WI 53597

SCALE 1" = .

	INDEX ROAD	169.14° (%) (%) (%) (%) (%) (%) (%) (%) (%) (%)	3.1500	Tro &	
,	101.2 SECTION 3-6-9 3,4" IRON REBAR LOI.2 C.S.M. 3505 C.S.M. 3505 C.S.M. 3505 OUTLOT 1 *2>/29.	26.Ft. LOT 3 124,367 Sq.Ft.	10 10 10 10 10 10 10 10 10 10 10 10 10 1	MAPLE LAWN HEIGHTS 32 25.	30
2728.81	1106.57 Segue Segu		Mezze	ned in 3 until Index ost Road is	and the agreed and 3.
N89'40'54"W	MINUM MONUMENT TON 3-6-9 MINUM MONUMENT FISH FISH FISH APPROPRIE APPROP	Nies rests	MAPLE LAWN HI	 Outlot 1 shall be owned and maintained in common by Lots 1, 2, and 3. There will be no development of Lot 3 unt Road is constructed to Post Road and Post Reconstructed to Post Road. 	3. The agreed maximum ISR is 65% and the minimum OSR is 35% for Lots 1, 2, and 3.



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(608) 825-2222 voice
(608) 825-2220 fex
www.transcend-arch.com

7AN. BEARING OUT S 41.54'24" W N 00'33'16" W

TAN. BEARING IN S 00°33'16" E N 41°54'30" E

BEARING S 20'40'34" W N 20'40'37" E

RADIUS 383.06 350.06

ARC 283.88 259.44

X0.

MAP

CERTIFIED SURVEY

DOCUMENT NO.

VOLUME

L:\2005\050421\070070_CSM J:\2005\050421 Sheet 1 of 5

Sheet 1 of 5 Map No. **070070_CSM**

AVALON SENIOR CAMPUS 2875 Fish Hatchery Road Fitchburg, Wisconsin 53713

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Overall CSM

Legend:
• = FOUND IRON STAKE P.O. Box 237 1677 N. Bristol Street Sun Prairie, Wl. 53590 Phone (608) 837–7463 Fax (608) 837–1081 BIRRENKOTT SURVEYING, INC.

CERTIFIED SURVEY MAP

PART OF LOT 21, LOTS 23 AND 24, AND PARTS OF LOTS 25, 27, AND 28, MAPLE LAWN HEIGHTS, ALL LOCATED IN THE NORTHEAST 1/4 OF THE NORTHEAST 1/4 OF SECTION 3, T6N, R9E, CITY OF FITCHBURG, DANE COUNTY, WISCONSIN

124,367 Sq.Ft. 70, VCCE22 EVEENL TO, VCCE22 EVEENL 58370'42"E N8758'06 N83.40,24,M. FINE OF THE NORTHEAST 1/4 BEARING BEARINGS REFERENCED TO THE NORTH **OUTLOT 1** 39.253 Sq.Ft. 3,92,64,92N N13'05'29"W \$ 15.00 +2.25N , 58.3 \$ <u>LOT 2</u> C.S.M. 3505 ·02 , 00, LOT 22 MAPLE LAWN HEIGHTS ٠٧چ 101 27 123 LOT 2 225,241 Sq.I BUILDING BUILDING O = SET 1"x24" IRON PIPE MIN. WGHT. 1.13 LBS/FT -STORY MESSESSM 100 11.6 3.1.5.9.5.66 3.1.5.9.5.66 1.2.0.6.6.7.N LOT 21 MAPLE LAWN HEIGHTS $\dot{\Diamond}$ ** LOT 1 C.S.M. 3505 Avalon Senior Care Bill Clemons 200 Meadow Oak Trail Waunakee, WI 53597 Ck.087 Prepared For: LOT 1 60,259 Sq.Ft. 50° -- 10

M.69, 7E.

.05

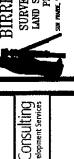
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PAGE CERTIFIED SURVEY MAP NO. DOCUMENT VOLUME

Overall CSM

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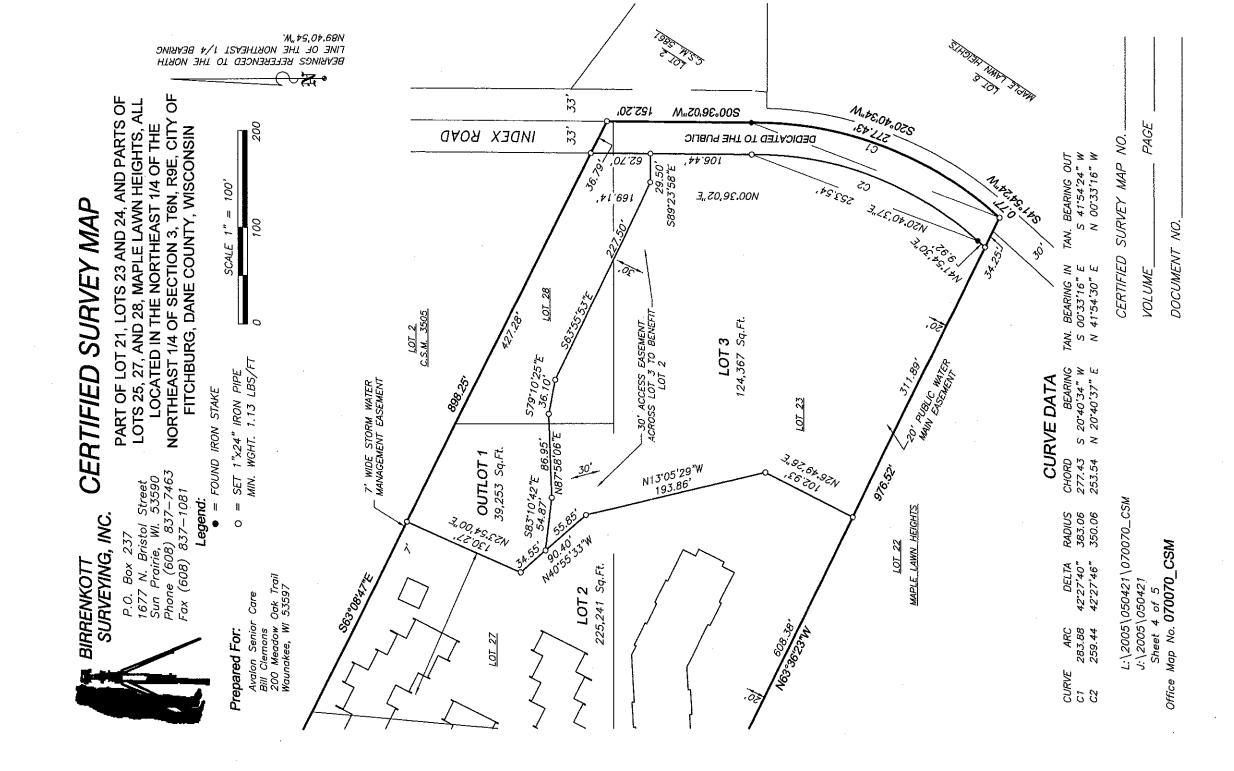
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LAND SURVEYING &
PERC TESTING
SUR FAMINE, INC. SING. (607) INT.

 ${f Transcend}$ Architects & Engine 1000 Lothe Street Jun Praide, Wi 53590

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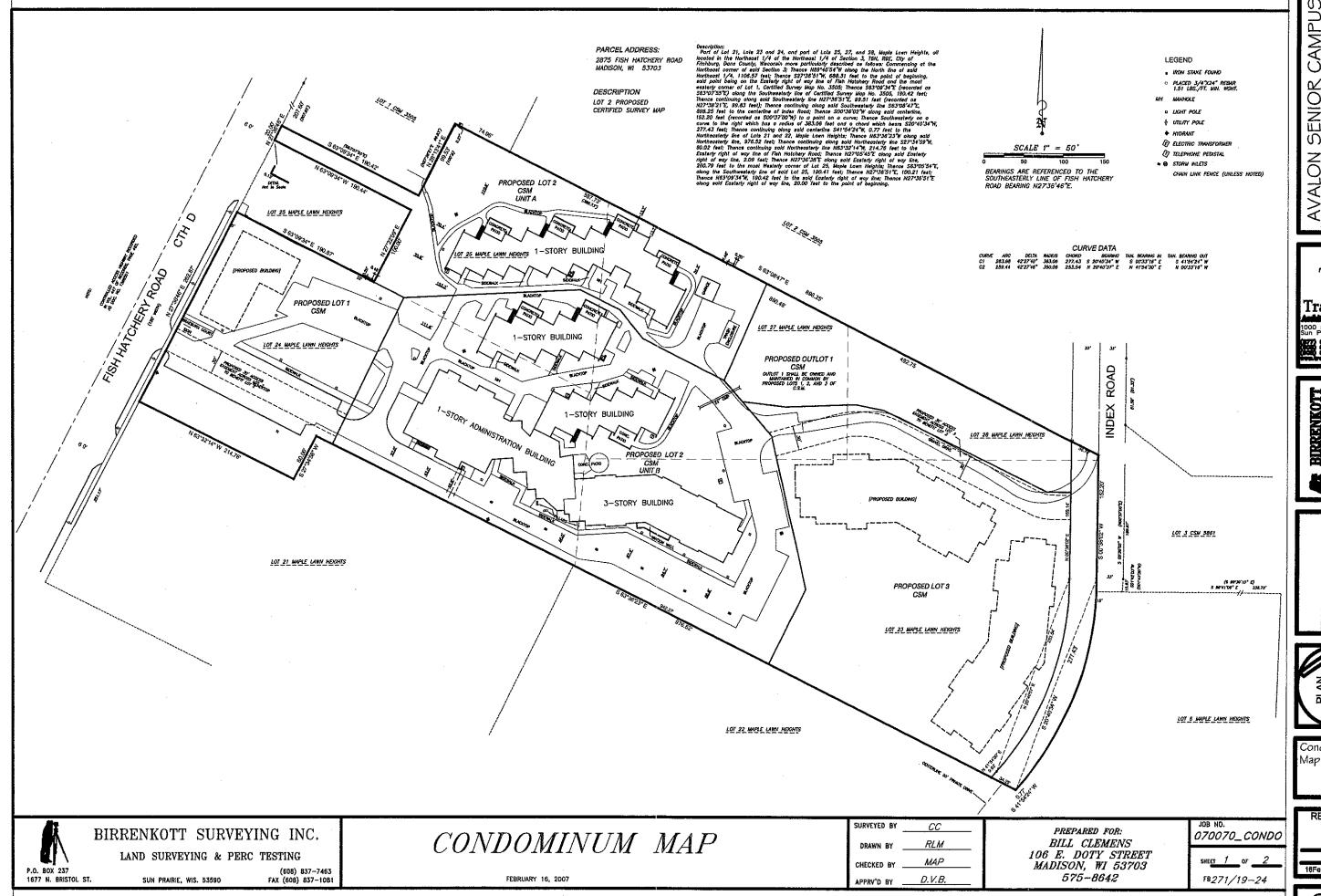
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Overall CSM

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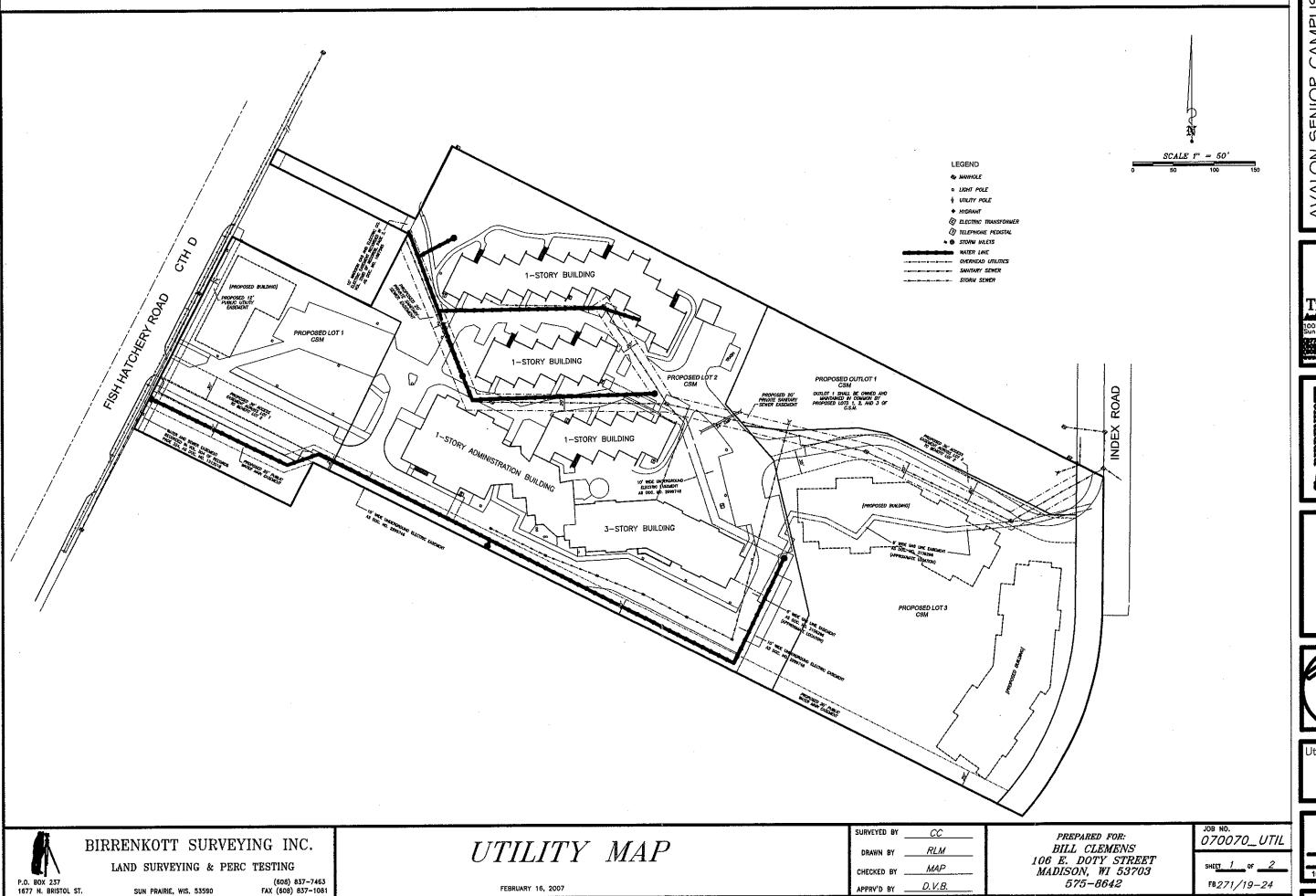
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Condominium Map

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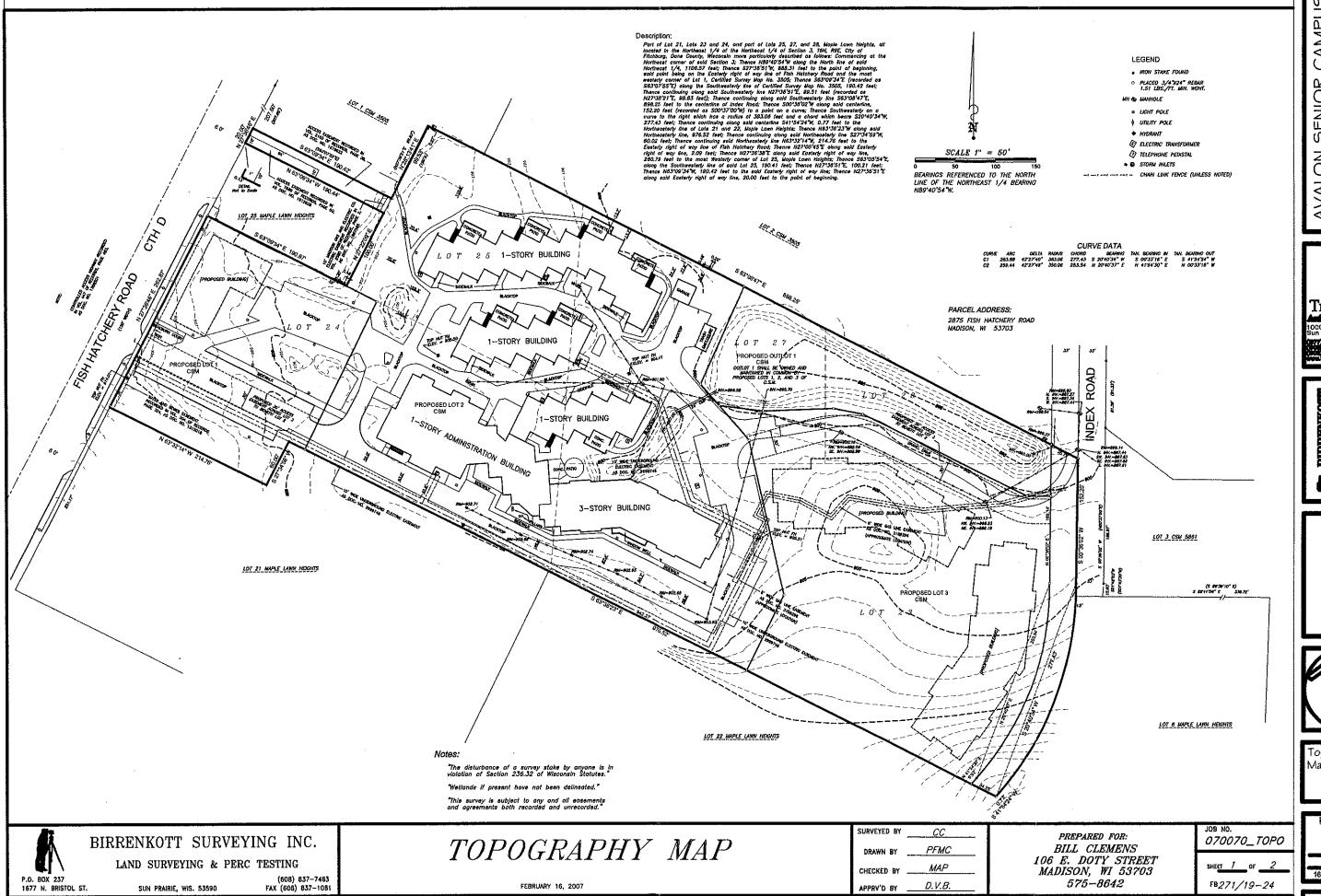
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Utility Map

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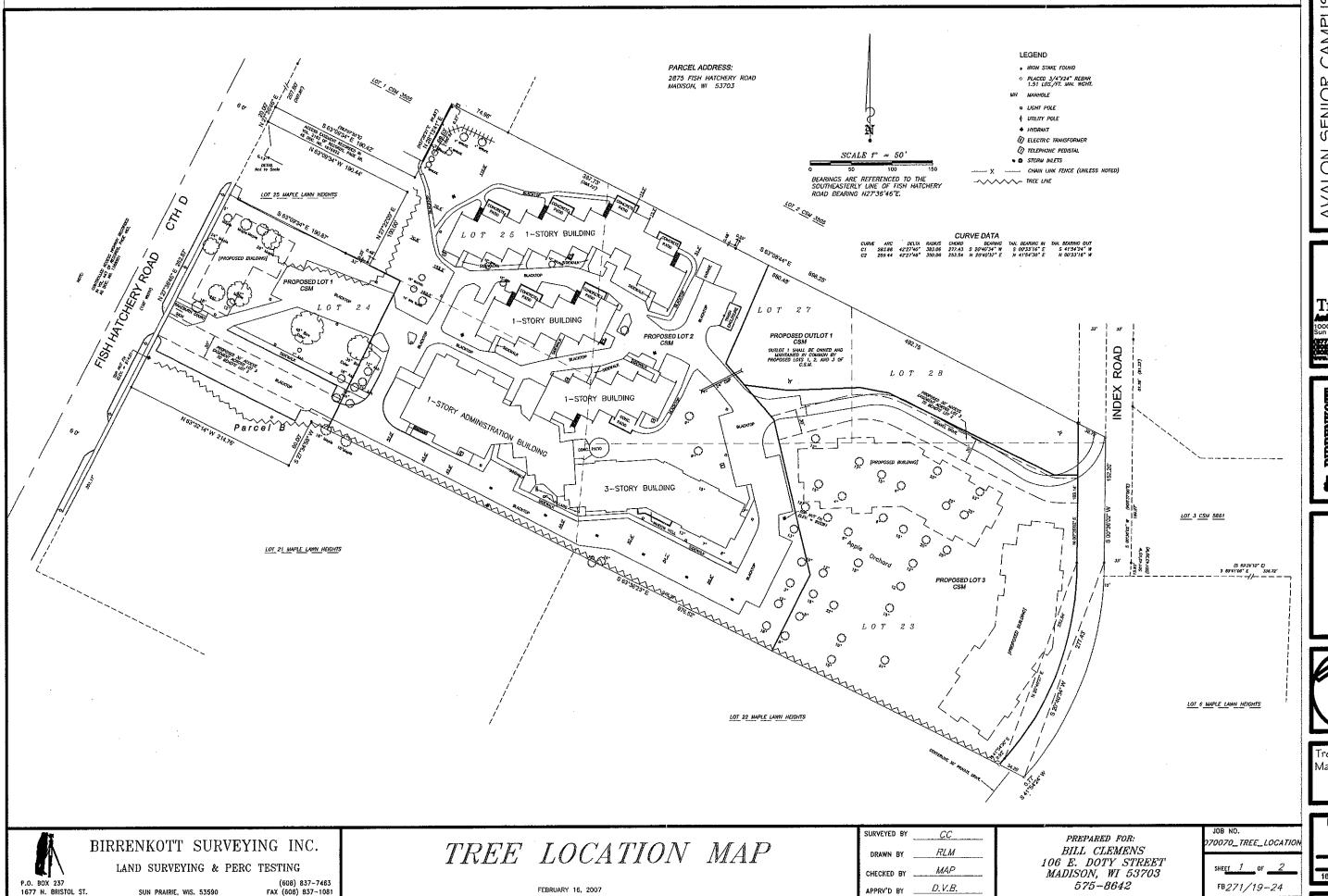
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Topography Map

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Sun Proirie, Wi 53590



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Tree Location Map

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